



LEARNING
CENTRE



Pad Footings

By

RK

Concrete Calculator

Sheet No.

1

Date

7/26

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Pad Footing Design Verification

This verification study benchmarks RKALC against an independently prepared design example for a reinforced concrete pad footing designed in accordance with AS 3600. The comparison covers flexural design in the critical direction together with column bearing, punching shear and one-way shear assessments. By verifying multiple limit states within a single example, the study demonstrates the accuracy and consistency of the RKALC design implementation for isolated footing design.



Materials

- $f'_c = 40 \text{ MPa}$
- $f_y = 500 \text{ MPa}$

Applied Loads

- $N_{DL} = 1500 \text{ kN}$
- $N_{LL} = 400 \text{ kN}$
- $N_{uls} = 2400 \text{ kN}$
- $N_{sls} = 1900 \text{ kN}$

Applied Stressess

- Bearing Capacity = 700 kPa

Applied soil stress = $1900 / (1.4 \times 2.1) = 646 \text{ kPa}$

Ultimate soil stress = $646 \times (2400 / 1900) = 816 \text{ kPa}$

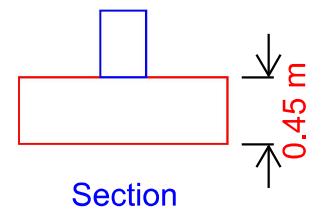
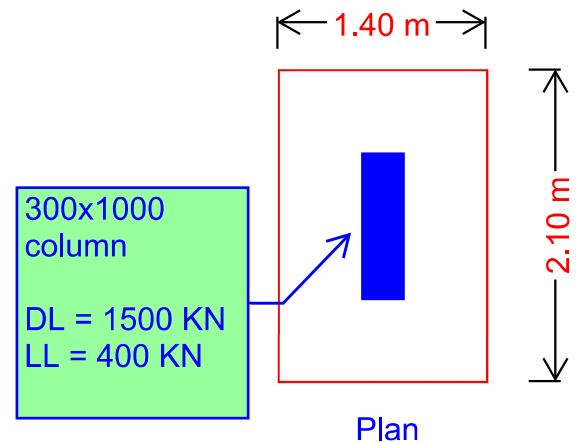
Outstand Checks

Cover to centre = 84 mm

Effective depth = $450 - 84 = 366 \text{ mm}$

Span-depth ratio = $550 / 366 = 1.503 > 1.5$

Hence footing can be designed as flexural member
[Ref. 12.1.1 AS 3600-2018]



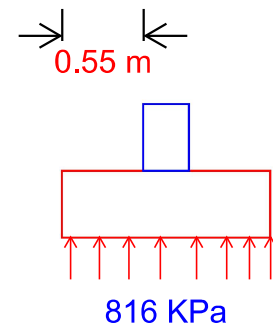
Flexural Design

$M^* = 816 \times (0.55^2 / 2) = 123 \text{ kN}\cdot\text{m}$ (per meter)

$A_{s \text{ min}} = 0.19 \times (D=450 / d=366)^2 \times (0.6 \times 3240 / 500)$ (see sec 9)

$A_s / bd = 0.218\% = 798 \text{ mm}^2$ ----Ref. 21.3.1

$A_{s \text{ Req}} = 123 \text{ E}+6 / 0.7 / 500 / 366 = 960$



Therefore

$A_s = \text{N16-200 EW}$

(Note: if the footing width is less than one metre, this 960 is to be provided within that width, or design the "beam" from first principles.)



Punching Shear Check

Critical shear perimeter

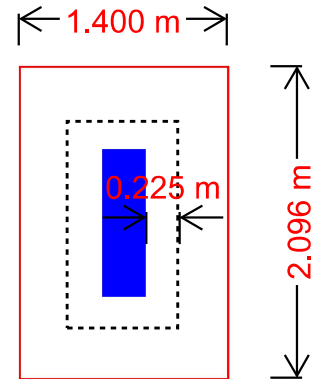
$$u = [(300+d)+(1000+d)] \times 2 = 4064 \text{ mm}$$

Punching reduction area @ $d/2$ =

$$((300+d)(1000+d) \times 816)/10^6 = 742 \text{ kN}$$

$$\text{therefore punching shear force} = 2400 - 742 = 1658 \text{ kN}$$

$$\text{Stress} = (1658 \times 10^3)/(4064 \times 366) = 1.115 \text{ MPa}$$



$$f_i F_{cv} = 0.7 \times 0.17 \times (1 + 2 \times 300/1000) \times 40^{0.5} = 1.204 \text{ MPa} \quad - \text{ref 9.3.3}$$

$$\text{Utilisation} = 1.115 / 1.205 = 93\%$$

One Way Shear

$$\text{Applied shear} = V^* = 816 \times 0.184$$

$$V^* = 150 \text{ kN}$$

$$\text{Stress} = (150 \times 10^3)/(1000 \times 326) = 0.46 \text{ MPa}$$

(note effective depth for shear is $0.72 \times$ Depth for one way shear)

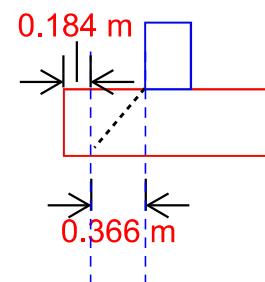
$$\epsilon = 0.000376$$

$$k_d = 0.888888$$

$$d_v = 326$$

$$\text{therefore } k_v = 0.257 \text{ (see clause 8.2.4.2.1)}$$

$$\text{therefore } v_c = 0.257 \times 40^{0.5} \times 0.7 = 1.137 \text{ MPa} > 0.46$$



NOTE: RKALC app still uses AS3600-2009 which is more conservative (gives allowable shear stress of 0.89 using Beta factors)



Column Bearing Check

Ref. 12.6

$$A1 = 300 \times 1000 = 300000$$

$$A2 = (300 + 2 \times 366)(1000 + 2 \times 366) = 1,787,424$$

$$(A2/A1)^{0.5} = 2.44 \text{ (max} = 2\text{)}$$

Stress @ A1 = 8 MPa

$$\text{Allowable stress} = 0.6 \times 0.9 \times 40 \times 2 = 43.20 \text{ MPa}$$

Development Length of Rebar (Column)

Rebar dia: say N16

$$\text{Compression length} > 10 \text{ dia} = 320 \text{ mm}$$



Pad Footings

Do Not Use for Span/Depth Ratio < 1.5

Use [STM Wall](#) for Footing on Rock

Footing Capacity

Calculations

My Footings

Inputs

Materials

f'_c (N/mm²)

40

f_y (N/mm²)

500

Geometry (Units mm)

Width

1400

Length

2100

Depth

450

Cover to Centre

84

Soil Bearing Capacity KPa

700

Column Width

300

Column Length

1000

Column's rebar size

16

Loads (KN , m)

Dead Load

1500

Live Load

400

Ultimate Load

2400

Calculate

RKALC Assistant New

Send to Database

General

Effective Depth

366

Span-Depth Ratio

1.5

Max Outstand

550

Soil Pressure

646

Flexure-Critical Dir.

Design Moment KN.m/m

123

Bottom REO (mm² / m)

950

Min Depth (Development)*

548

**(anchorage length of Column reo that is needed for section capacity at face of footing, exercise judgement)*

Column Bearing

Stress under Col (A1)

8

Max Bearing Stress

43.2

Bearing Utilisation

19 %

Punching Shear

Punching Stress

1.11

Conc Punching Capacity

1.2

Punching Utilisation

93 %

One Way Shear

(AS3600-2009 formulae 8.2.7.1)

Applied Shear KN/m

150

Applied Shear MPa/m

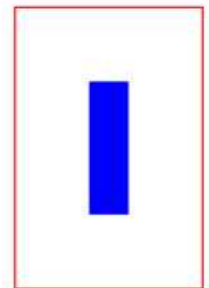
0.41

Shear Capacity MPa

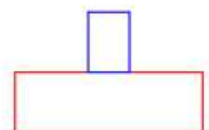
0.89

Graphics

Plan



Section





Pad Footing Design - AS3600-2018

Flexural, bearing, punching shear and one-way shear assessment

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Calculations by
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1. Introduction

This document provides analysis and design of Pad Footing in accordance with the Australian Standard AS3600-2009.

The following outcomes are provided:

- Flexural Design of Critical Direction
- Column Bearing Check
- Punching Shear Check
- One Way Shear Check

The design ensures structural integrity and compliance with the standard under the specified loading conditions. For detailed guidelines and requirements, refer to the relevant sections of AS3600-2018-A2.

This calculation can be reproduced and checked independently. For more details, refer to the [RKALC website](#).

2. Material Properties

Property	Value	Unit
Concrete Strength f_c	40	MPa
Rebar f_y	500	MPa

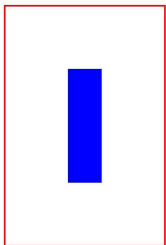
3. Geometry and Applied Loads

Geometry

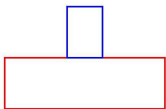
Geometry Parameter	Value	Unit
Footing Width	1400	
Footing Length	2100	
Footing Depth	450	
Cover to Centre of Reo	84	
Soil Bearing Capacity	700	
Column Width	300	
Column Length	1000	
Column Rebar Size	16	

Applied Loads

Load Parameter	Value	Unit
DL	1500	KN
LL	400	KN
Ultimate Load	2400	KN



Footing Plan



Footing Section

4. Results

General

General Result	Value	Unit
Effective Depth	366	KN
Span-Depth Ratio	1.5	KN
Max Outstand	550	mm ²
Soil Pressure (SL5)	646	KN

Fexural Check - Critical Direction

Flexural Result	Value	Unit
Design Moment	123	KN.m/m
Bottom REO	950	(mm ² / m)
Min Depth (Development)	548	mm

Column Bearing Check

Bearing Result	Value	Unit
Stress under Col (A1) - Clause 12.6	8	MPa
Max Bearing Stress	43.2	MPa
Bearing Utilisation	19	%

Punching Shear Check

Punching Shear Result	Value	Unit
Punching Stress	1.11	MPa
Conc Punching Capacity	1.2	MPa
Punching Utilisation	93	%

One Way Shear Check - (AS3600-2009 formulae 8.2.7.1)

One Way Shear Result	Value	Unit
Applied Shear	150	KN/m
Applied Shear	0.41	MPa/m
Shear Ccapacity	0.89	MPa

note RKALC still adopts 2009 standard here for simplicity, this will be changed after the 2026 new standard is mandated in NCC